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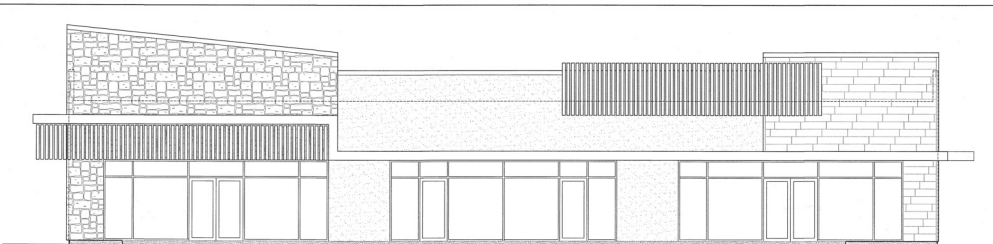
REVISED ISSUE DATES:
8/12/2014
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POTRANCO SHELL BUILDING
8815 POTRANCO RD.
SAN ANTONIO, TX, 78251



PROJECT NO. 14-023
DATE: 07.07.14
DRAWN BY: CS
REVIEWED BY: JH
PROJECT ARCHITECT:
DURAND-HOLLIS RUPE ARCHITECTS, P.C.
TEXAS LICENSE NO. 10881

COVER



GENERAL NOTES

- THIS SET OF CONSTRUCTION DOCUMENTS IS PRESENTED TO INCLUDE DRAWINGS OF 1/4" = 3/8" SHEETS & A SEPARATE BOUND SET OF SPECIFICATIONS.
- UNLESS NOTED OTHERWISE, ELECTRICAL, CONDUIT, CABLES, ETC. SHALL BE RUN CONCEALED AND FINISHES SHALL BE AS INDICATED. DIMENSIONS TO ACCOMMODATE THIS SHALL BE THE RESPONSIBILITY OF THE ARCHITECT.
- FOR ANY ITEM IDENTIFIED IN THE CONSTRUCTION DOCUMENTS THAT IS REASONABLY ANTICIPATED AS A COMPONENT OF A SYSTEM AND REQUIRED FOR THE PERFORMANCE OF THAT SYSTEM, THE CONTRACTOR SHALL INCLUDE ALL CODE COMPONENTS IN THE WORK WHICH ARE NECESSARY FOR THE COMPLETE AND FULLY OPERATIONAL PERFORMANCE OF THAT SYSTEM.
- ALL INFORMATION ON EXISTING CONDITIONS WAS OBTAINED FROM DURAND-HOLLIS RUPE ARCHITECTS BY THE OWNER. CONTRACTOR IS REQUESTED TO VERIFY, CORRECT, AND/OR AMEND AS NECESSARY BEFORE BEGINNING CONSTRUCTION. CONTRACTOR SHALL FURNISH A REPORT TO DURAND-HOLLIS RUPE ARCHITECTS, CONTRACTOR SHALL FURNISH A NEW (REV) SET WITH EXISTING CONDITIONS PRIOR TO COMMENCING CONSTRUCTION.
- WORK CONTAINED IN THESE PLANS AND SPECIFICATIONS RELATE TO NEW FLOOR, WALL, DOOR, FRAMES, CGL FINISHES AND CABINETS AS SCHEDULED OR NOTED.
- REPAIR ANY DAMAGED WALL PRIOR TO APPLYING FINISHES.
- CONTRACTOR TO PROVIDE AND INSTALL 6-18 BATT INSULATION ON ALL EXTERIOR WALLS ABOVE SCHEDULED FINISH AS SPECIFIED BY MANUFACTURER.
- THE CONSTRUCTION DOCUMENTS ARE COMPLEMENTARY, AND WHAT IS REQUIRED BY ONE SHALL BE AS REQUIRED BY ALL. ALL CONTRACT DOCUMENTS, INCLUDING SPECIFICATIONS, SHALL BE USED TO OBTAIN COMPLETE SETS OF DOCUMENTS AND REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO THE START OF CONSTRUCTION.
- FOR ANY ITEM IDENTIFIED IN THE CONSTRUCTION DOCUMENTS THAT IS REASONABLY ANTICIPATED AS A COMPONENT OF A SYSTEM AND REQUIRED FOR THE PERFORMANCE OF THAT SYSTEM, THE CONTRACTOR SHALL INCLUDE ALL CODE COMPONENTS IN THE WORK WHICH ARE NECESSARY FOR THE COMPLETE AND FULLY OPERATIONAL PERFORMANCE OF THAT SYSTEM.
- THE CONTRACT DOCUMENTS INDICATE THE GENERAL DESIGN INTENT, BUT DO NOT NECESSARILY DEFINE ALL WORK REQUIRED FOR FULL PERFORMANCE AND COMPLETION. THE CONTRACTOR SHALL PROVIDE ALL ITEMS REQUIRED FOR THE PROPER EXECUTION AND COMPLETION OF THE WORK.
- CONTRACTOR OF THE WORK SHALL VERIFY IN THE FIELD AND COORDINATE WITH THE OWNER, ALL CONDITIONS WITH ANY AND EXISTING WORK NOT AFFECTED BY THE WORK. ALL CONDITIONS INCLUDING, BUT NOT LIMITED TO, EXISTING WORK, UTILITIES, EXISTING STRUCTURAL CONDITIONS, EXISTING LOCATIONS OF ALL CONSTRUCTION, EXISTING AND NEW EXISTENCE AND LOCATIONS OF ACCESSORIES OR OTHER HAZARDOUS MATERIALS, OBSTRUCTIONS, WALLS, FLOORS, VENTILATION, DRAINAGE, AND OTHER CONDITIONS. CONTRACTOR IS RESPONSIBLE FOR THE DISCOVERY OF ACCESSORIES AND OTHER REGULATED ITEMS, OBSTRUCTIONS, AND HAZARDOUS MATERIALS. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DISCOVERY OF HAZARDOUS MATERIALS. SHOULD ANY CONDITIONS BE FOUND TO REQUIRE CONSTRUCTION OR REMOVAL OF ANY PART OF CONSTRUCTION, CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT FOR REVIEW AND CLARIFICATION BEFORE PROCEEDING WITH THAT PORTION OF THE WORK OR ANY PART RELATED HERETO.
- CONTRACTOR SHALL OBTAIN AND BE RESPONSIBLE FOR ALL FEES AND PERMITS REQUIRED AND ASSOCIATED WITH ALL PHASES OF THE WORK AND WITHIN SCOPE OF THE CONSTRUCTION DOCUMENTS INCLUDING, BUT NOT LIMITED TO, BUILDING, FIRE, FLOOD, WATER AND SEWER FEES, GRADING AND EROSION CONTROL FEES, AND LOCAL, STATE AND FEDERAL PERMITS. CONTRACTOR SHALL BE RESPONSIBLE FOR THE LOCATION OF ALL UTILITIES BEFORE STARTING CONSTRUCTION.
- CONTRACTOR TO PROVIDE ALL REQUIRED BUILDINGS FOR INSTALLATION OF SYSTEMS SUCH AS, BUT NOT LIMITED TO, DRAIN, DRAINAGE, SHELTER, ETC.

SYMBOLS

KEY	DESCRIPTION
SHEET NUMBER (TOTAL NUMBER)	DETAIL KEY
ROOM	ROOM NAME AND NUMBER
DOOR	DOOR SCHEDULE NUMBER
WINDOW	WINDOW KEY
DEMOLITION KEYNOTE	DEMOLITION KEYNOTE
NEW WORK AND KEY NOTES	NEW WORK AND KEY NOTES
ELEVATION KEY	ELEVATION KEY
SECTION KEY	SECTION KEY
DETAIL KEY	DETAIL KEY
PARTITION KEY	PARTITION KEY
TRUE NORTH ARROW	TRUE NORTH ARROW
PLAN NORTH ARROW	PLAN NORTH ARROW
FINISH KEY	FINISH KEY

CODE INFORMATION

CODES	REQUIREMENT CLASSIFICATION
2012 INTERNATIONAL BUILDING CODE	B-BUSINESS
2012 INTERNATIONAL FIRE CODE	V-S
2009 INTERNATIONAL ENERGY CONSERVATION CODE	OCCUPANT LOAD (OCC)
2012 UNIFORM PLUMBING CODE	PLUMBING COUNT
2012 INTERNATIONAL MECHANICAL CODE	MECHANICAL COUNT
2011 NATIONAL ELECTRICAL CODE	ELECTRICAL COUNT

PROJECT LOCATION



PROJECT TEAM

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STRUCTURAL ENGINEER DATTU ENGINEERS 5001 BRIDGEMAN SAN ANTONIO, TX 78209 P: (210) 808-2880 LAUREN@DATTUENGINEERS.COM	GEOTECHNICAL ARIAS & ASSOCIATES 110 CALLE 100 SAN ANTONIO, TX 78232 P: 210.308.3889 F: 210.308.3889 JERRY@ARIAS-PA.COM
MECH., ELEC., AND PLUMB. ENGINEER MURRAY & ASSOCIATES 10100 SAN PETER AVENUE SAN ANTONIO, TX 78216 P: (210) 930-3000 BEE@MURRAY-PA.COM DANIEL@MURRAY-PA.COM	
LANDSCAPE ARCHITECT HORIZON DESIGN AND DEVELOPMENT 132 RIO BRITO ST. SAN ANTONIO, TEXAS 78232 P: (210) 831-8544 JOHN@HORIZONDESIGN-PA.COM	

DRAWING LIST

GENERAL INFORMATION
COVER PROJECT INFORMATION, CODE ANALYSIS, LOCATION MAP
CIVIL
C301 GENERAL NOTES
C302 EXISTING CONDITIONS
C303 CONSTRUCTION PLAN
C304 CIVIL SITE PLAN
C305 SITE CONSTRUCTION PLAN
C306 SITE CONSTRUCTION AND FIRE PROTECTION PLAN
C307 SITE CONSTRUCTION PLAN
C308 SITE CONSTRUCTION AND FIRE PROTECTION PLAN
C309 SITE DETAILS 1 OF 2
C310 SITE DETAILS 2 OF 2
ARCHITECTURAL
A301 SITE PLAN
A302 FLOOR PLAN & REVOLVED CEILING PLAN
A303 ROOF PLAN
A304 ELEVATIONS
A305 BUILDING SECTIONS
A306 WALL SECTIONS
A307 WALL SECTIONS
A308 DOOR & WINDOW TYPES
A309 DETAILS
STRUCTURAL
S301 TYPICAL ABBREVIATION SYMBOLS AND PLAN NOTES
S302 GENERAL NOTES
S303 FOUNDATION PLAN
S304 TYPICAL SLAB ON GRADE DETAILS
S305 TYPICAL CONCRETE SECTIONS AND DETAILS
S306 TYPICAL STEEL DETAILS
S307 TYPICAL STEEL DETAILS
S308 SWAYE BEAM CONNECTION DETAILS
S309 ROOF SECTION DETAILS
MEP
M301 PLUMBING SYMBOLS
M302 PLUMBING PLAN
M303 PLUMBING DETAILS
ELECTRICAL
E301 ELECTRICAL SYMBOLS
E302 SITE ELECTRICAL PLAN
E303 ELECTRICAL POWER PLAN
E304 ELECTRICAL ONE LINE DIAGRAM
E305 ELECTRICAL SPECIFICATIONS
LANDSCAPE ARCHITECT
L301 LANDSCAPE PLAN
L302 LANDSCAPE SPECIFICATIONS
L303 IRRIGATION DETAILS AND PLAN
L304 IRRIGATION DETAILS

WOOD FRAMING

- UNLESS OTHERWISE INDICATED, WOOD FRAMING SHALL COMPLY WITH SECTION 2008 "CONVENTIONAL LIGHT FRAME CONSTRUCTION" AND TABLE 2008.1.1 "TYPICAL REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE." THE CONTRACTOR SHALL MAINTAIN A COPY FOR REFERENCE AT THE JOBSITE. WALLS SHALL BE COMMON WALLS 2x6 D.
- JOISTS, BATTERS AND BEARING WALLS SHALL BE NO. 2 SOUTHERN PINE OR NO. 2 DOUGLASS FIR. NON-BEARING WALL FRAMING BY THE CONTRACTOR (WIDE SPACING: PINE-FIR (SP)).
- PROVIDE SILL BLOCKING ON A BIRD BOARD AT ALL FLOOR JOIST BEARING LOCATIONS OVER WALLS, BEAMS, STUDS OR HEADERS. BLOCKING SHALL BE SAME DEPTH AS JOIST. LAP JOISTS AT 24" MINIMUM OVER SUPPORTS.
- TE OPPOSING BATTERS TOGETHER AT 4'-0" ON CENTER BY NAILING TO ADJACENT CEILING JOISTS OR BY INSTALLING 1x6 (MINIMUM) CROSS TIES AT LEAST 2'-0" BELOW ROOF ELEVATION.
- PLACE SINGLE PLATE AT THE BOTTOM AND A DOUBLE PLATE AT THE TOP OF ALL STUD WALLS. OFFSET STUDS 4'-0" IN TOP PLATE AND OVERLAP AT CORNERS. EXTERIOR SILL PLATES SHALL BE 2x6 TYP. TO THE JOIST OR 2x12 TYP. IF JOIST IS 12" MINIMUM. AT A MAXIMUM OF 4'-0" ON CENTER. INTERIOR SILL PLATES SHALL BE 2x6 OR 2x12 TYP. TO THE FOUNDATION AT 4'-0" ON CENTER. THERE SHALL BE ONE ANCHOR WITHIN 12" OF EACH END OF EACH PIECE.
- SILL PLATES BEARING ON FOUNDATION OR IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESURE-TREATED WITH PRESERVATIVE.
- STUDS SHALL BE DOUBLED AT ALL ANGLES, CORNERS AND AROUND ALL OPENINGS.
- ALL EXTERIOR CORNERS SHALL BE BRACED WITH 1x6x8x8 BRACE OF 1x6 APA RATED SHEATHING WITH AN EXPOSURE 1 RATING. METAL VERTICALS ON THE EXTERIOR FACE WITH ONE INSET 1/4" EDGE OF THE CORNER.
- UNLESS OTHERWISE SHOWN ON PLANS, WOOD UNITS OVER OPENINGS SHALL BE 2x12 TYP. 2x8 HEADERS FOR SPANS UNDER 4'-0" OR SPANS FROM 4'-0" TO 7'-0" WOOD UNITS SHALL BE 2x12 TYP. 2x8 HEADERS FOR SPANS FROM 7'-0" TO 10'-0" WOOD UNITS SHALL BE 2x12 TYP. 2x8 HEADERS.
- UNLESS OTHERWISE DETAILED FOR FLOOR OR ROOF JOIST CONNECTIONS TO SUPPORTING BEAM, FLASH TYPE CONNECTIONS, USE TYPE "L" JOIST HANGERS AS MANUFACTURED BY THE SIMPSON COMPANY OR "TODOLAP" JOIST HANGERS AS MANUFACTURED BY THE HANCOCK COMPANY. FOR ROOF JOIST HANGERS, JOIST HANGERS SHALL BE TYPE "TSL" AS MANUFACTURED BY THE SIMPSON COMPANY OR "EQS" AS MANUFACTURED BY THE HANCOCK COMPANY. THE TYPE OF HANGER USED SHALL BE AS RECOMMENDED BY THE MANUFACTURER FOR THE SIZE OF JOIST SUPPORTED.
- W/TO BEAM SHALL BE BOLTED TOGETHER WITH 3/4" DIAMETER BOLTS TOP AND BOTTOM OVER SUPPORT OR AT ENDS OF BEAM. THEN 1/2" DIAMETER BOLTS AT 2'-0" ON CENTER. STANDARDIZED FULL LENGTH OF BEAM. STEEL PLATES FOR STEEL BEAMS SHALL CONFORM TO THE LATEST AISI SPECIFICATION AND MATERIALS SHALL CONFORM TO ASTM A36.
- UNLESS OTHERWISE NOTED ON THE DRAWINGS, ALL CONTINUOUS JOISTS SHALL EXTEND INTO THE BUILDING A DISTANCE EQUAL TO THE CONTINUOUS JOIST. CONTINUOUS JOISTS RUNNING PERPENDICULAR TO FRAMING INSIDE THE BUILDING SHALL BE CONNECTED TO DOUBLE END JOISTS BY NAILING. JOISTS RUNNING PARALLEL TO FRAMING INSIDE THE BUILDING SHALL BE CONTINUOUS. EXTENSIONS OF JOIST MEMBERS ON LAPPS 4'-0" AND NAILS TO THE SIDE OF JOIST MEMBERS WITH NO WALL AT 1'-0" ON CENTER, TOP AND BOTTOM.
- PROVIDE DOUBLE JOISTS UNDER ALL INTERIOR PARTITION WALLS WHICH RUN PARALLEL TO JOISTS (AND ARE NOT SUPPORTED BY ALKALINE WALLS BELOW).
- THE STUDS IN THE WALLS SHALL BE CONTINUOUS FROM THE FLOOR TO THE MAXIMUM LEVEL OF FRAMING (ROOF, CEILING JOISTS OR FLOOR), UNLESS DETAILED OTHERWISE. DO NOT INTERRUPT STUD FRAMING WITH AN INTERMEDIATE HEAD PLATE IN WALL WALLS. USE FULL HEIGHT STUDS.
- ALL BATTENS AND SCOFFS SHALL HAVE STANDARD HANGERS. ALL ANCHOR AND EXPANSION BOLTS USED FOR WOOD TO CONCRETE CONNECTIONS IN THE CRACK SPACE SHALL BE HOT DIP GALVANIZED OR STAINLESS STEEL.
- REFER TO THE ARCHITECTURAL DRAWINGS FOR ADDITIONAL WOOD FRAMING MEMBERS AND PROVIDE SUCH MEMBERS EVEN THOUGH NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- DO NOT BEAR JOISTS OR BEAMS DIRECTLY ON CONCRETE OR MASONRY. SET JOISTS AND BEAMS ON 2x6 SILL PLATES. PRESURE-TREATED WITH PRESERVATIVE.
- ROOF SHEATHING SHALL BE 1/2" APA RATED SHEATHING WITH AN EXPOSURE 1 RATING. PANELS SHALL BE CONTINUOUS OVER TWO OR MORE SPANS WITH THE LONG DIMENSION ORIENTED PERPENDICULAR TO THE FRAMING MEMBERS. PROVIDE 1/4" GAP BETWEEN SHEATHING PANELS FOR ALL USES. EXTEND SHEATHING THROUGH BRANCH OVERLAP AREAS TO COMPLETE THE DIAPHRAGM.
- CONNECTION HARDWARE: ALL METAL CONNECTIONS AND STRAPS SHALL BE FURNISHED WITH GALVANIZED STEEL. ALL CONNECTION ASSEMBLIES FABRICATED FROM STEEL STRUCTURAL SHAPES AND PLATES SHALL BE HOT DIP GALVANIZED AFTER FABRICATION. FASTENERS USED IN EXTERIOR LOCATIONS SHALL BE GALVANIZED.

PREFABRICATED METAL PLATE CONNECTED WOOD TRUSSES

- CONTRACTOR SHALL USE PREFABRICATED METAL PLATE CONNECTED WOOD TRUSSES IN lieu of engineered wood joists and beams, OR SOLID SAWN LUMBER, UNLESS NOTED OTHERWISE ON THE DRAWINGS. NOTED OTHERWISE ON THE DRAWINGS.
- PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER. THE TRUSSES SHALL BE DESIGNED, FABRICATED AND NUTTED TO COMPLY WITH THE FOLLOWING STANDARDS AND SPECIFICATIONS:

- "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" PUBLISHED BY THE NATIONAL WOOD PRODUCT ASSOCIATION.
- AMERICAN NATIONAL DESIGN STANDARD FOR METAL PLATE-CONNECTED WOOD TRUSS CONSTRUCTION.
- "TIP-108" COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES.
- "TIP-108" RECOMMENDED DESIGN SPECIFICATION FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES.
- "WCTC" BUILDING COMPONENT SAFETY INFORMATION FOR 140.

WHERE THERE IS A CONFLICT BETWEEN THE TWO PUBLICATIONS, THE MORE STRINGENT REQUIREMENT SHALL GOVERN.

- TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED TO WITHSTAND THE ROOF LIVE LOADS AND WIND LOADS INDICATED IN THE GENERAL NOTES AND AS FOLLOWS:

CEILING 2 PSF

- UNLESS OTHERWISE NOTED ON THE DRAWINGS, THE ROOF SHALL BE DESIGNED TO WITHSTAND THE ROOF LIVE LOADS AND WIND LOADS INDICATED IN THE GENERAL NOTES AND AS FOLLOWS:

WIND OR SEISMIC LOADS 25 PERCENT

WIND OR SEISMIC LOADS 35 PERCENT

- THE TRUSS MANUFACTURER SHALL SUBMIT THE FOLLOWING CERTIFICATIONS, SEALED BY THE ENGINEER RESPONSIBLE FOR DESIGN, FOR THE ARCHITECT'S RECORD PRIOR TO FABRICATION OF ANY TRUSSES:

- CERTIFICATION OF THE TRUSS MANUFACTURER'S DESIGN TO SECURE THE MEMBERS, BY AN INDEPENDENT AGENCY.
- CERTIFICATION THAT THE MANUFACTURER IS LICENSED TO FABRICATE TRUSSES USING THE CONNECTION SYSTEM PROPOSED.
- CERTIFICATION THAT THE TRUSSES ARE DESIGNED TO MEET THE LOAD CRITERIA SPECIFIED HEREIN.
- FABRICATION AND INSTALLATION DRAWINGS SHOWING SIZES, SHAPES, AND LAYOUT.

- TOP AND BOTTOM CHORDS SHALL BE DESIGNED TO RESIST BENDING INDUCED BY THE LOADS IMPOSED BETWEEN PANEL JOINTS, INCLUDING CONNECTION LOADS. WEB MEMBERS SHALL BE DESIGNED TO SPAN FULL LENGTH BETWEEN CHORDS WITHOUT INTERMEDIATE BRACINGS.

- DEFLECTIONS DUE TO LIVE LOAD SHALL BE LIMITED TO L/80.

- BOTTOM CHORDS SHALL NOT BE SPACED WITHIN THE MIDDLE THIRD OF THE SPAN LENGTH.

- CONNECT ROOF TRUSSES TO BEARING WALL OR BEAM SUPPORT AT EACH END, WITH A TYPE AS FRAMING ANCHOR, AS MANUFACTURED BY SIMPSON COMPANY OR APPROVED EQUAL.

- TRUSSES AT THE ENDS OF BUILDINGS OR AT STEP DOWNS IN THE ROOF, DETAILED TO RESIST EXTERIOR BENDING OR KINKING, ARE TO BE PROVIDED WITH AN VERTICAL STUDS AT 1'-0" ON CENTER, SHIP FABRICATED ONTO THE TRUSSES. VERIFY WITH ARCHITECTURAL IF FRAMED OPENINGS AND SPECIAL VENTS ARE REQUIRED.

- PROVIDE SPECIAL BEARINGS, IF REQUIRED, TO ALLOW TRUSSES TO BUTT AT INTERIOR SUPPORT CONDITIONS.

- THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING TEMPORARY ERECTION BRACING, AS REQUIRED, UNTIL PERMANENT BRACING AND A SUFFICIENT PORTION OF THE ROOF DECK HAS BEEN INSTALLED TO ADEQUATELY BRACE THE TRUSSES.

- THE TRUSS SUPPLIER SHALL PROVIDE BOTTOM CHORD BRACING, AS REQUIRED, WHERE A GYPSUM BOARD CEILING DOES NOT CONNECT DIRECTLY TO THE BOTTOM CHORDS.

- ALL CONNECTION PLATES, HANGERS AND OTHER HARDWARE SHALL BE GALVANIZED. FASTENERS USED IN EXPOSED EXTERIOR LOCATIONS SHALL BE GALVANIZED OR EPOXY COATED.

- PROVIDE 2x4 DIAGONAL BRACINGS AT GABLE ENDS TO BRACE THE BOTTOM CHORD BETWEEN INTERSECTING WALLS.



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REVISION/ISSUE DATES:
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POTRANCO SHELL BUILDING
4015 POTRANCO RD.
SAN ANTONIO, TX 78211



DATE: 1/28/2014



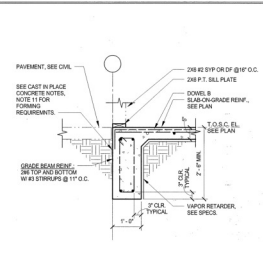
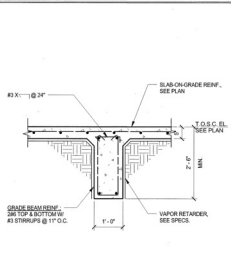
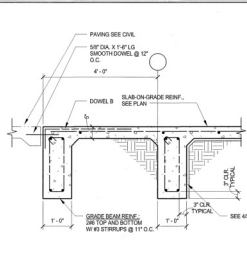
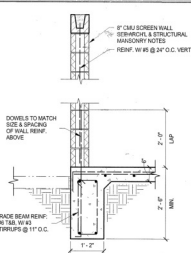
DATE: 1/28/2014

PROJECT NO. 14-003
DRAWN BY: LR
REVIEW BY: LR
PROJECT ARCHITECT:
GABRIEL DURAN-HOLLIS, FAIA

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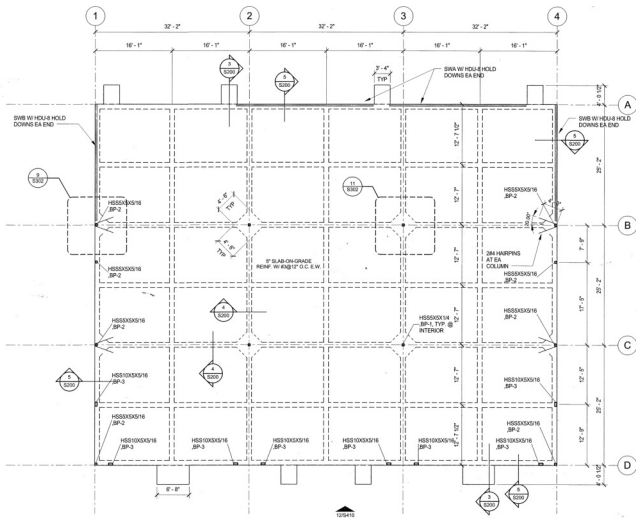
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REVISION/ISSUE DATES:
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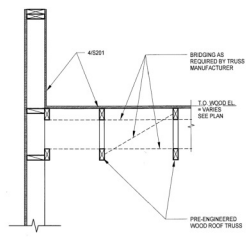
STRUCTURAL MASONRY

- MASONRY DESIGN IS BASED ON A MASONRY FLEXURE STRENGTH OF $f_m = 1500$ PSI.
- HOLLOW CLAY MASONRY UNITS SHALL CONFORM TO ASTM C-90, LIGHTWEIGHT, GRADE N, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1800 ON THE NET AREA OF THE BLOCK.
- MORTAR SHALL CONFORM TO ASTM C-270, TYPE S, CONSISTING OF 1 PART PORTLAND CEMENT, 1 1/2 PARTS MASONRY SAND, AND 3 PARTS MASONRY SAND. DO NOT USE AIR ENTRAINING LIME. MASONRY CEMENT ON MASONRY SAND.
- CONCRETE GROUT SHALL CONFORM TO ASTM C-476, WITH A MAXIMUM AGGREGATE SIZE OF 3/8". PROPORTIONS BY VOLUME SHALL BE: 1 PART PORTLAND CEMENT, 2 1/4 PARTS MASONRY SAND, AND 1 1/2 PARTS FINE GRIT. SLUMP SHALL BE 3" TO 4".
- VERTICAL REINFORCING IS SHOWN ON THE DETAILS. IN ADDITION, THE FIRST CELL AT CORNERS AND ENDS OF WALLS SHALL BE REINFORCED WITH 4# AND GROUTED. MAX. EXTENDING 2' BEYOND CORNERS SHALL BE PLACED EACH SIDE OF WALL OPENINGS.
- PROVIDE FOUNDATION CORNERS TO MATCH MASONRY WALL REINFORCEMENT. CORNERS SHALL EXTEND A MINIMUM OF 8" BAR DIAMETERS ABOVE AND 30" BAR DIAMETERS BELOW TOP OF FOUNDATION.
- VERTICAL BARS SHALL BE HELD IN POSITION IN THE CENTER OF THE CELL OR GROUT DURING PLACEMENT OF EACH 1' OF GROUT UNLESS ANOTHER POSITION IS DETAILED.
- PROVIDE WEIR HOLES AT 2' ON CENTER IN VERTICAL JOINTS OF EXTERIOR WALLS OVER ALL GROUTED BOND BEAMS, DOOR HEADERS, OR OTHER CONTINUOUS OBSTRUCTIONS WITHIN THE Cavity AND ALONG FOUNDATION BEAMS.
- WHEN A FOUNDATION CORNER DOES NOT LIE UP WITH A VERTICAL CORNER, IT SHALL BE SLOPED NOT MORE THAN ONE HORIZONTAL TO SIX VERTICAL. THE CORNER SHALL BE GROUTED INTO A CELL IN VERTICAL ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCEMENT.
- HORIZONTAL REINFORCING BARS SHALL BE PLACED IN CONTINUOUS MASONRY COURSES, CONSISTING OF BOND BEAM OR THROUGH BLOCK CAVITY ARE SHALL BE GROUTED IN PLACE.
- REINFORCING BARS SHALL BE STRAIGHT EXCEPT FOR BENDS AROUND CORNERS AND WHERE BENDS OR HOOKS ARE DETAILED ON THE PLANS.
- HORIZONTAL REINFORCING STEEL SHALL BE LAPPED WHERE SPICES AND SHALL BE WELDED TOGETHER. BEND BARS AT CORNERS AND TEE INTERSECTIONS PROVIDING STANDARD LAP SPICES WITH INTERSECTING BARS.
- STANDARD LAP SPICES LENGTHS FOR REINFORCING BARS SHALL BE AS FOLLOWS:
#5 BAR DIAMETERS FOR BARS AND SMALLER
#6 BAR DIAMETERS FOR BARS AND SMALLER
#8 BAR DIAMETERS FOR BARS AND SMALLER
#10 BAR DIAMETERS FOR BARS AND SMALLER
- HORIZONTAL WALL JOINT REINFORCING SHALL BE STANDARD CUT "SUN-O-RAM" (TRUSS TYPE) AT 18" ON CENTER, VERTICALLY. TYPICAL WALLS SEE NOTES OVERLAP.
- STANDARD WIRE REINFORCEMENT SHALL BE LAPPED AT LEAST 1' AT SPICES AND SHALL CONTAIN AT LEAST ONE CROSS WIRE OF EACH PIECE OF REINFORCEMENT IN THE LAP JOINT.
- VERTICAL REINFORCING BARS MAY BE SPICED IN 6" TO 12" (12") LENGTHS, PROVIDED THE SPICES IN ADJACENT WALLS ARE REINFORCED WITH 4# AND GROUTED SO THAT NOT MORE THAN ONE-HALF OF THE TOTAL NUMBER OF BARS ARE SPICED AT ANY ONE HEIGHT.

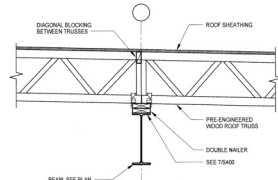


PLAN NOTES:

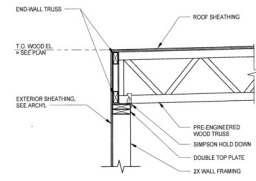
- NORTH ARROW
- SEE SHEET S100 FOR TYPICAL PLAN NOTES, SYMBOLS, ABBREVIATIONS AND TYPICAL DETAIL SHEET LIST.
- SEE SHEET S101 FOR GENERAL NOTES.



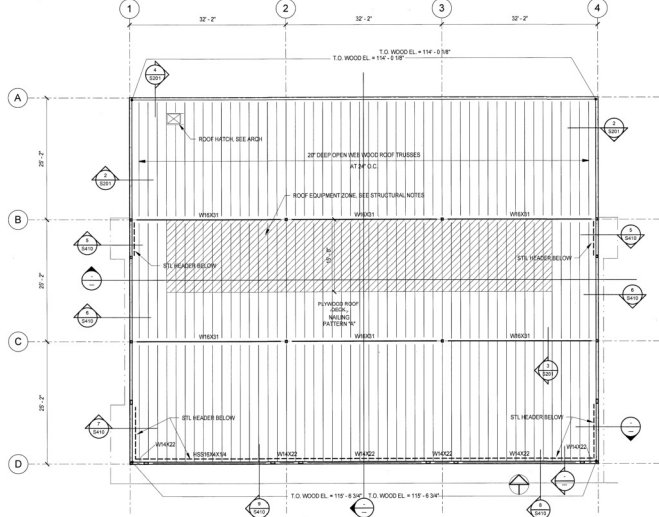
1" = 1'-0" 2



1" = 1'-0" 3



1" = 1'-0" 4



5 ROOF FRAMING PLAN
1/8" = 1'-0"

- PLAN NOTES:**
1. NORTH ARROW
 2. SEE SHEET S101 FOR TYPICAL PLAN NOTES, SYMBOLS, ABBREVIATIONS AND TYPICAL DETAIL SHEET LISTS
 3. SEE SHEET S101 FOR GENERAL NOTES

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Fax: 210.897.3335
Email: info@datumtx.com



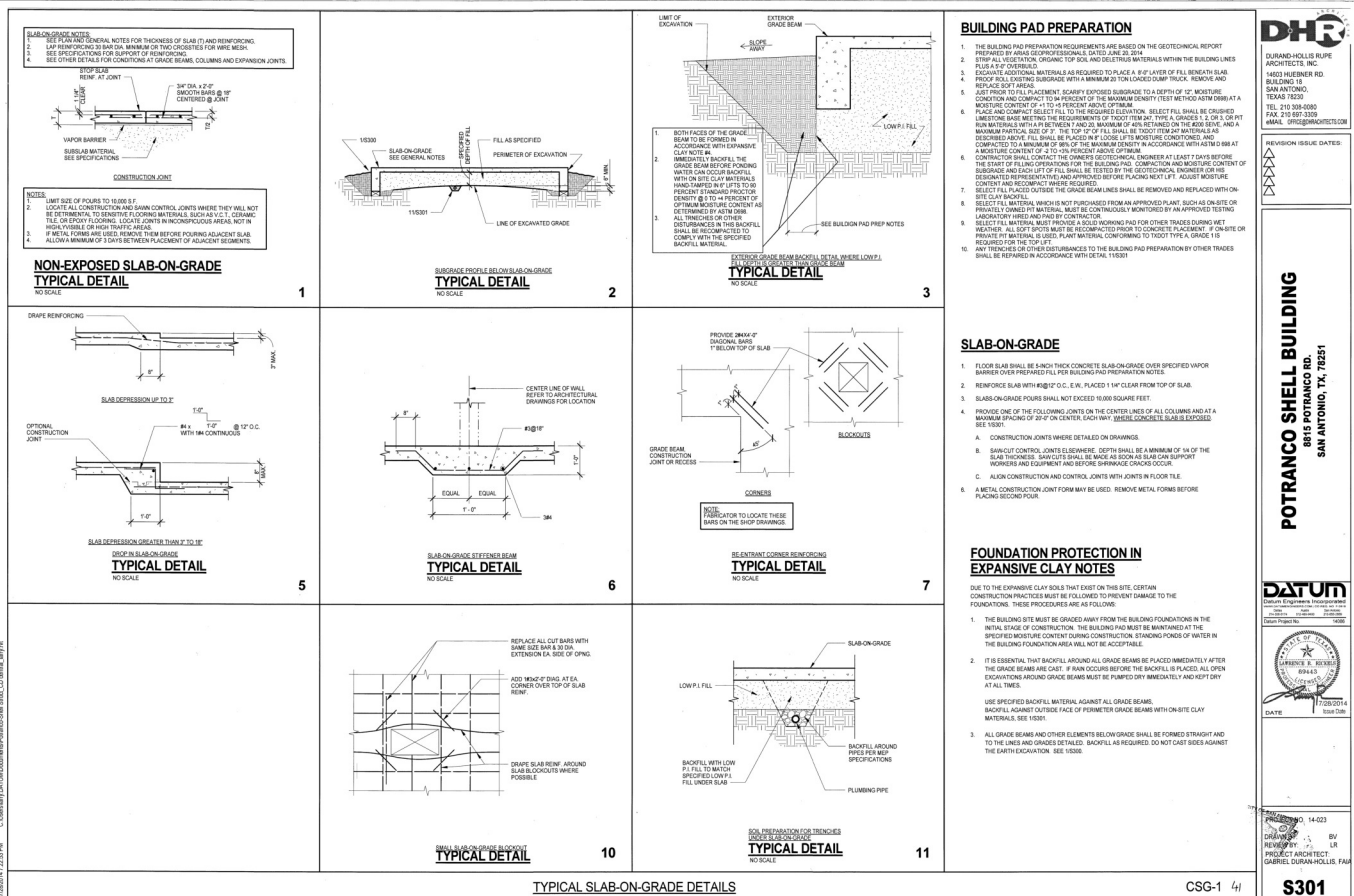
DATE: 12/20/2014 Issue Date

PROJECT NO. 14-023
DRAWN BY: [Signature]
REVIEWED BY: [Signature]
PROJECT ARCHITECT:
GABRIEL DURAND-HOLLIS, FAIA

S201

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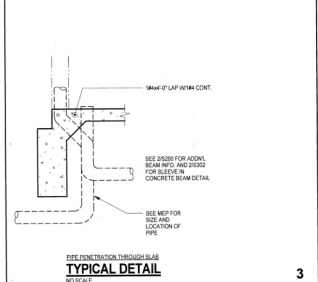
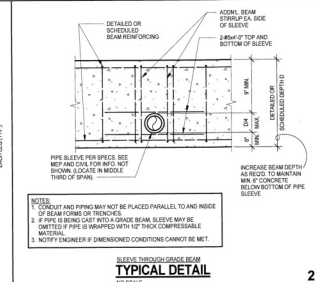
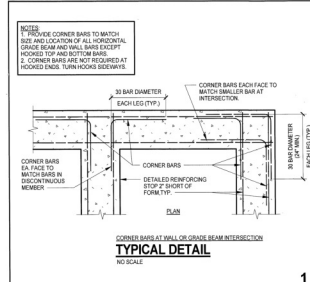


TYPICAL SLAB-ON-GRADE DETAILS

CSG-1 44

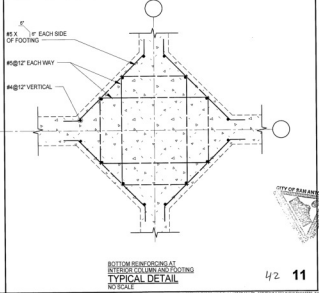
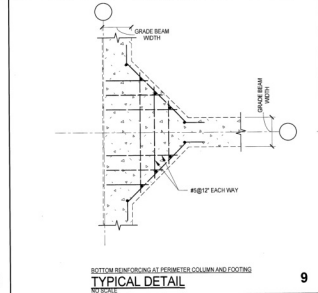
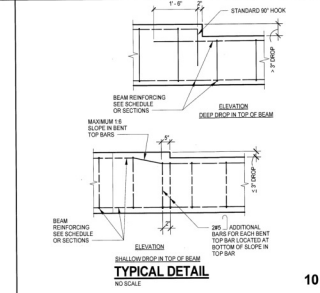
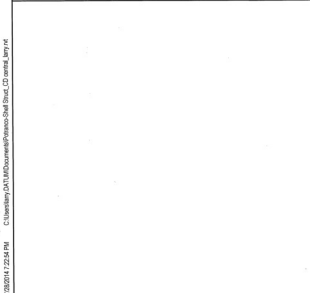
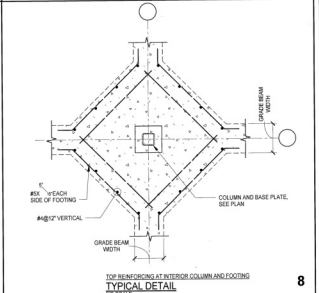
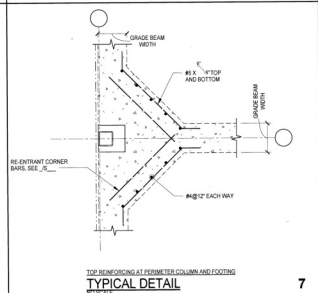
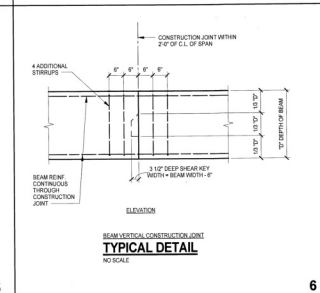
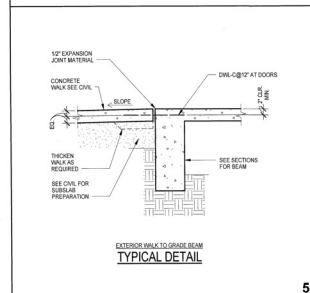
REVISIONS (DATE)

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DOWEL SCHEDULE				
MARK	SIZE	A	B	C
DWL-A	#4	8"	3'0"	—
DWL-B	#4	3'0"	1'0"	—
DWL-C	#4	—	4'0"	—
DWL-D	#5 THD	—	4'0"	—
DWL-E	#4 THD	—	7'0"	—
DWL-F	#4 THD	—	8'0"	—

NOTES:
1. SCHEDULED DOWELS ARE MARKED "DWL" ON THE SECTIONS AND DETAILS.
2. DOWEL SPACING TO BE THE SAME AS VERTICAL BEAM OR WALL REINFORCEMENT UNLESS OTHERWISE NOTED ON THE WALL DOWELS WITH TOP A TEST COLUMN SHALL BE THREADED DOWELS WITH APPROVED DOWEL BAR ANCHORS PER SPECIFICATION.
4. FOR TILT WALL PANELS USE 4" LITZ.



DHR
DURAND HOLLIS RUPE
ARCHITECTS, INC.
14803 HUEBNER RD.
SAN ANTONIO,
TEXAS 78253
TEL 210.505.0080
FAX 210.507.3038
EMAIL: OFFICE@DHRARCHITECTS.COM

REVISION/ISSUE DATES:
A
B
C

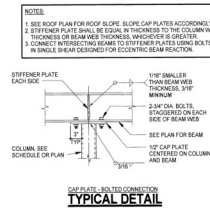
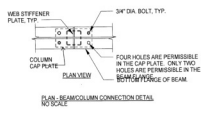
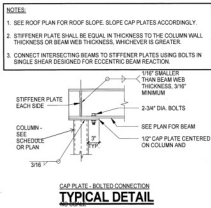
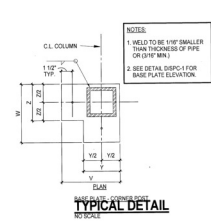
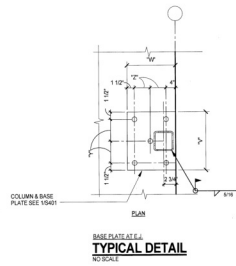
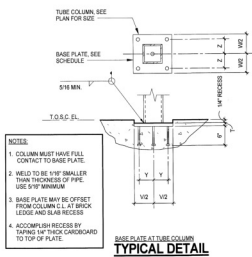
POTRANCO SHELL BUILDING
8815 POTRANCO RD.
SAN ANTONIO, TX, 78251

DATUM
Professional Engineer's Seal
GABRIEL DURAND HOLLIS
170000014
DATE: 7/26/2014
BY: GDR

PROJECT NO. 14-023
BY: LR
DRAWN BY: LR
REVIEWED BY: LR
PROJECT ARCHITECT:
GABRIEL DURAND HOLLIS, FAIA

S302

BASE PLATE SCHEDULE						
MARK	V	W	Y	Z	T	NUMBER, SIZE & SCHEDULED EMBEDMENT LENGTH OF A, B, DETAIL NUMBER
BP-1	11"	11"	4"	4"	34"	(4) - 3/4" X 1'-0" HCA 25500
BP-2	11"	13.5"	4"	4"	34"	(5) - 3/4" X 1'-0" HCA 25500
BP-3	14"	13.5"	4"	4"	34"	(5) - 3/4" X 1'-0" HCA 25500
BP-4						
BP-5						
BP-6						
BP-7						
BP-8						



DHR
 DURAND-HOLLIS RUPPE
 ARCHITECTS, INC.
 14603 HUEBNER RD.
 SAN ANTONIO,
 TEXAS 78245
 TEL. 210.308.0080
 FAX. 210.697.3339
 EMAIL: JTR@DHRARCHITECTS.COM

REVISION ISSUE DATES:
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ANGLES WELDED TO BEAM

SEE ANGLES BOLTED TO BEAM OR FLANGE
BOLTED TO BEAM OR FLANGE

DOUBLE ANGLES
EACH SIDE OF THE
3/4" DIA. BOLTS
EACH SIDE FOR 7/8" DIA. BOLTS

CONNECTION LOAD CAPACITY

WIDE FLANGE BEAM SIZE	# BOLTS PER ROW	MINIMUM BEAM WEB THICKNESS (IN.)				MAX REACTION (KIPS)				MIN. ANGLE LENGTH (IN.)
		3/4" DIA. BOLTS	7/8" DIA. BOLTS	1" DIA. BOLTS	1 1/4" DIA. BOLTS					
W8 TO W8	2	230	29	29	14	13	4	5 1/2		
W8 TO W10	3	230	31	31	26	23	4	5 1/2		
W8 TO W12	4	230	33	33	42	39	5	11 1/2		
W8 TO W14	5	230	35	35	63	59	6	14 1/2		
W8 TO W16	6	230	37	37	84	80	7	17 1/2		
W8 TO W18	7	230	39	39	105	100	8	20 1/2		
W8 TO W20	8	230	41	41	126	120	9	23 1/2		
W8 TO W22	9	230	43	43	147	140	10	26 1/2		
W8 TO W24	10	230	45	45	168	160	11	29 1/2		
W8 TO W26	11	230	47	47	189	180	12	32 1/2		
W8 TO W28	12	230	49	49	210	200	13	35 1/2		

NOTE: VALUES IN THE COLUMNS LABELED "BOLTED TO BEAM" MAY BE INCREASED BY 1/4" DIA. OF THE BEAM WEB THICKNESS TO THE MINIMUM BEAM WEB THICKNESS TABULATED. FOR THIS CALCULATION THE BEAM WEB THICKNESS MAY NOT BE TAKEN MORE THAN 1/4" DIA. OF THE BEAM WEB THICKNESS FOR 7/8" DIA. BOLTS.

CONNECTION LOAD CAPACITY (X = 3" MAX.)

WIDE FLANGE BEAM SIZE	# BOLTS PER ROW	MINIMUM BEAM WEB THICKNESS (IN.)				MAX REACTION (KIPS)				MIN. ANGLE LENGTH (IN.)
		3/4" DIA. BOLTS	7/8" DIA. BOLTS	1" DIA. BOLTS	1 1/4" DIA. BOLTS					
W8 TO W8	2	230	29	29	14	13	4	5 1/2		
W8 TO W10	3	230	31	31	26	23	4	5 1/2		
W8 TO W12	4	230	33	33	42	39	5	11 1/2		
W8 TO W14	5	230	35	35	63	59	6	14 1/2		
W8 TO W16	6	230	37	37	84	80	7	17 1/2		
W8 TO W18	7	230	39	39	105	100	8	20 1/2		
W8 TO W20	8	230	41	41	126	120	9	23 1/2		
W8 TO W22	9	230	43	43	147	140	10	26 1/2		
W8 TO W24	10	230	45	45	168	160	11	29 1/2		
W8 TO W26	11	230	47	47	189	180	12	32 1/2		
W8 TO W28	12	230	49	49	210	200	13	35 1/2		

NOTE: LARGER (IN) REACTION MUST BE LESS THAN 1/4" DIA. OF THE BEAM WEB THICKNESS, 3/4" DIA. BOLTS.

CONNECTION LOAD CAPACITY (X = 3" MAX.)

WIDE FLANGE BEAM SIZE	# BOLTS PER ROW	MINIMUM BEAM WEB THICKNESS (IN.)				MAX REACTION (KIPS)				MIN. ANGLE LENGTH (IN.)
		3/4" DIA. BOLTS	7/8" DIA. BOLTS	1" DIA. BOLTS	1 1/4" DIA. BOLTS					
W8 TO W8	2	230	29	29	14	13	4	5 1/2		
W8 TO W10	3	230	31	31	26	23	4	5 1/2		
W8 TO W12	4	230	33	33	42	39	5	11 1/2		
W8 TO W14	5	230	35	35	63	59	6	14 1/2		
W8 TO W16	6	230	37	37	84	80	7	17 1/2		
W8 TO W18	7	230	39	39	105	100	8	20 1/2		
W8 TO W20	8	230	41	41	126	120	9	23 1/2		
W8 TO W22	9	230	43	43	147	140	10	26 1/2		
W8 TO W24	10	230	45	45	168	160	11	29 1/2		
W8 TO W26	11	230	47	47	189	180	12	32 1/2		
W8 TO W28	12	230	49	49	210	200	13	35 1/2		

NOTE: LARGER (IN) REACTION MUST BE LESS THAN 1/4" DIA. OF THE BEAM WEB THICKNESS, 3/4" DIA. BOLTS.

CONNECTION LOAD CAPACITY (X = 3" MAX.)

WIDE FLANGE BEAM SIZE	# BOLTS PER ROW	MINIMUM BEAM WEB THICKNESS (IN.)				MAX REACTION (KIPS)				MIN. ANGLE LENGTH (IN.)
		3/4" DIA. BOLTS	7/8" DIA. BOLTS	1" DIA. BOLTS	1 1/4" DIA. BOLTS					
W8 TO W8	2	230	29	29	14	13	4	5 1/2		
W8 TO W10	3	230	31	31	26	23	4	5 1/2		
W8 TO W12	4	230	33	33	42	39	5	11 1/2		
W8 TO W14	5	230	35	35	63	59	6	14 1/2		
W8 TO W16	6	230	37	37	84	80	7	17 1/2		
W8 TO W18	7	230	39	39	105	100	8	20 1/2		
W8 TO W20	8	230	41	41	126	120	9	23 1/2		
W8 TO W22	9	230	43	43	147	140	10	26 1/2		
W8 TO W24	10	230	45	45	168	160	11	29 1/2		
W8 TO W26	11	230	47	47	189	180	12	32 1/2		
W8 TO W28	12	230	49	49	210	200	13	35 1/2		

NOTE: LARGER (IN) REACTION MUST BE LESS THAN 1/4" DIA. OF THE BEAM WEB THICKNESS, 3/4" DIA. BOLTS.

CONNECTION LOAD CAPACITY (X = 3" MAX.)

WIDE FLANGE BEAM SIZE	# BOLTS PER ROW	MINIMUM BEAM WEB THICKNESS (IN.)				MAX REACTION (KIPS)				MIN. ANGLE LENGTH (IN.)
		3/4" DIA. BOLTS	7/8" DIA. BOLTS	1" DIA. BOLTS	1 1/4" DIA. BOLTS					
W8 TO W8	2	230	29	29	14	13	4	5 1/2		
W8 TO W10	3	230	31	31	26	23	4	5 1/2		
W8 TO W12	4	230	33	33	42	39	5	11 1/2		
W8 TO W14	5	230	35	35	63	59	6	14 1/2		
W8 TO W16	6	230	37	37	84	80	7	17 1/2		
W8 TO W18	7	230	39	39	105	100	8	20 1/2		
W8 TO W20	8	230	41	41	126	120	9	23 1/2		
W8 TO W22	9	230	43	43	147	140	10	26 1/2		
W8 TO W24	10	230	45	45	168	160	11	29 1/2		
W8 TO W26	11	230	47	47	189	180	12	32 1/2		
W8 TO W28	12	230	49	49	210	200	13	35 1/2		

NOTE: LARGER (IN) REACTION MUST BE LESS THAN 1/4" DIA. OF THE BEAM WEB THICKNESS, 3/4" DIA. BOLTS.

CONNECTION LOAD CAPACITY (X = 3" MAX.)

WIDE FLANGE BEAM SIZE	# BOLTS PER ROW	MINIMUM BEAM WEB THICKNESS (IN.)				MAX REACTION (KIPS)				MIN. ANGLE LENGTH (IN.)
		3/4" DIA. BOLTS	7/8" DIA. BOLTS	1" DIA. BOLTS	1 1/4" DIA. BOLTS					
W8 TO W8	2	230	29	29	14	13	4	5 1/2		
W8 TO W10	3	230	31	31	26	23	4	5 1/2		
W8 TO W12	4	230	33	33	42	39	5	11 1/2		
W8 TO W14	5	230	35	35	63	59	6	14 1/2		
W8 TO W16	6	230	37	37	84	80	7	17 1/2		
W8 TO W18	7	230	39	39	105	100	8	20 1/2		
W8 TO W20	8	230	41	41	126	120	9	23 1/2		
W8 TO W22	9	230	43	43	147	140	10	26 1/2		
W8 TO W24	10	230	45	45	168	160	11	29 1/2		
W8 TO W26	11	230	47	47	189	180	12	32 1/2		
W8 TO W28	12	230	49	49	210	200	13	35 1/2		

NOTE: LARGER (IN) REACTION MUST BE LESS THAN 1/4" DIA. OF THE BEAM WEB THICKNESS, 3/4" DIA. BOLTS.

CONNECTION LOAD CAPACITY (X = 3" MAX.)

WIDE FLANGE BEAM SIZE	# BOLTS PER ROW	MINIMUM BEAM WEB THICKNESS (IN.)				MAX REACTION (KIPS)				MIN. ANGLE LENGTH (IN.)
		3/4" DIA. BOLTS	7/8" DIA. BOLTS	1" DIA. BOLTS	1 1/4" DIA. BOLTS					
W8 TO W8	2	230	29	29	14	13	4	5 1/2		
W8 TO W10	3	230	31	31	26	23	4	5 1/2		
W8 TO W12	4	230	33	33	42	39	5	11 1/2		
W8 TO W14	5	230	35	35	63	59	6	14 1/2		
W8 TO W16	6	230	37	37	84	80	7	17 1/2		
W8 TO W18	7	230	39	39	105	100	8	20 1/2		
W8 TO W20	8	230	41	41	126	120	9	23 1/2		
W8 TO W22	9	230	43	43	147	140	10	26 1/2		
W8 TO W24	10	230	45	45	168	160	11	29 1/2		
W8 TO W26	11	230	47	47	189	180	12	32 1/2		
W8 TO W28	12	230	49	49	210	200	13	35 1/2		

NOTE: LARGER (IN) REACTION MUST BE LESS THAN 1/4" DIA. OF THE BEAM WEB THICKNESS, 3/4" DIA. BOLTS.

CONNECTION LOAD CAPACITY (X = 3" MAX.)

WIDE FLANGE BEAM SIZE	# BOLTS PER ROW	MINIMUM BEAM WEB THICKNESS (IN.)				MAX REACTION (KIPS)				MIN. ANGLE LENGTH (IN.)
		3/4" DIA. BOLTS	7/8" DIA. BOLTS	1" DIA. BOLTS	1 1/4" DIA. BOLTS					
W8 TO W8	2	230	29	29	14	13	4	5 1/2		
W8 TO W10	3	230	31	31	26	23	4	5 1/2		
W8 TO W12	4	230	33	33	42	39	5	11 1/2		
W8 TO W14	5	230	35	35	63	59	6	14 1/2		
W8 TO W16	6	230	37	37	84	80	7	17 1/2		
W8 TO W18	7	230	39	39	105	100	8	20 1/2		
W8 TO W20	8	230	41	41	126	120	9	23 1/2		
W8 TO W22	9	230	43	43	147	140	10	26 1/2		
W8 TO W24	10	230	45	45	168	160	11	29 1/2		
W8 TO W26	11	230	47	47	189	180	12	32 1/2		
W8 TO W28	12	230	49	49	210	200	13	35 1/2		

NOTE: LARGER (IN) REACTION MUST BE LESS THAN 1/4" DIA. OF THE BEAM WEB THICKNESS, 3/4" DIA. BOLTS.

CONNECTION LOAD CAPACITY (X = 3" MAX.)

WIDE FLANGE BEAM SIZE	# BOLTS PER ROW	MINIMUM BEAM WEB THICKNESS (IN.)				MAX REACTION (KIPS)				MIN. ANGLE LENGTH (IN.)
		3/4" DIA. BOLTS	7/8" DIA. BOLTS	1" DIA. BOLTS	1 1/4" DIA. BOLTS					
W8 TO W8	2	230	29	29	14	13	4	5 1/2		
W8 TO W10	3	230	31	31	26	23	4	5 1/2		
W8 TO W12	4	230	33	33	42	39	5	11 1/2		
W8 TO W14	5	230	35	35	63	59	6	14 1/2		
W8 TO W16	6	230	37	37	84	80	7	17 1/2		
W8 TO W18	7	230	39	39	105	100	8	20 1/2		
W8 TO W20	8	230	41	41	126	120	9	23 1/2		
W8 TO W22	9	230	43	43	147	140	10	26 1/2		
W8 TO W24	10	230	45	45	168	160	11	29 1/2		
W8 TO W26	11	230	47	47	189	180	12	32 1/2		
W8 TO W28	12	230	49	49	210	200	13	35 1/2		

NOTE: LARGER (IN) REACTION MUST BE LESS THAN 1/4" DIA. OF THE BEAM WEB THICKNESS, 3/4" DIA. BOLTS.

CONNECTION LOAD CAPACITY (X = 3" MAX.)

WIDE FLANGE BEAM SIZE	# BOLTS PER ROW	MINIMUM BEAM WEB THICKNESS (IN.)				MAX REACTION (KIPS)				MIN. ANGLE LENGTH (IN.)
		3/4" DIA. BOLTS	7/8" DIA. BOLTS	1" DIA. BOLTS	1 1/4" DIA. BOLTS					
W8 TO W8	2	230	29	29	14	13	4	5 1/2		
W8 TO W10	3	230	31	31	26	23	4	5 1/2		
W8 TO W12	4	230	33	33	42	39	5	11 1/2		
W8 TO W14	5	230	35	35	63	59	6	14 1/2		
W8 TO W16	6	230	37	37	84	80	7	17		



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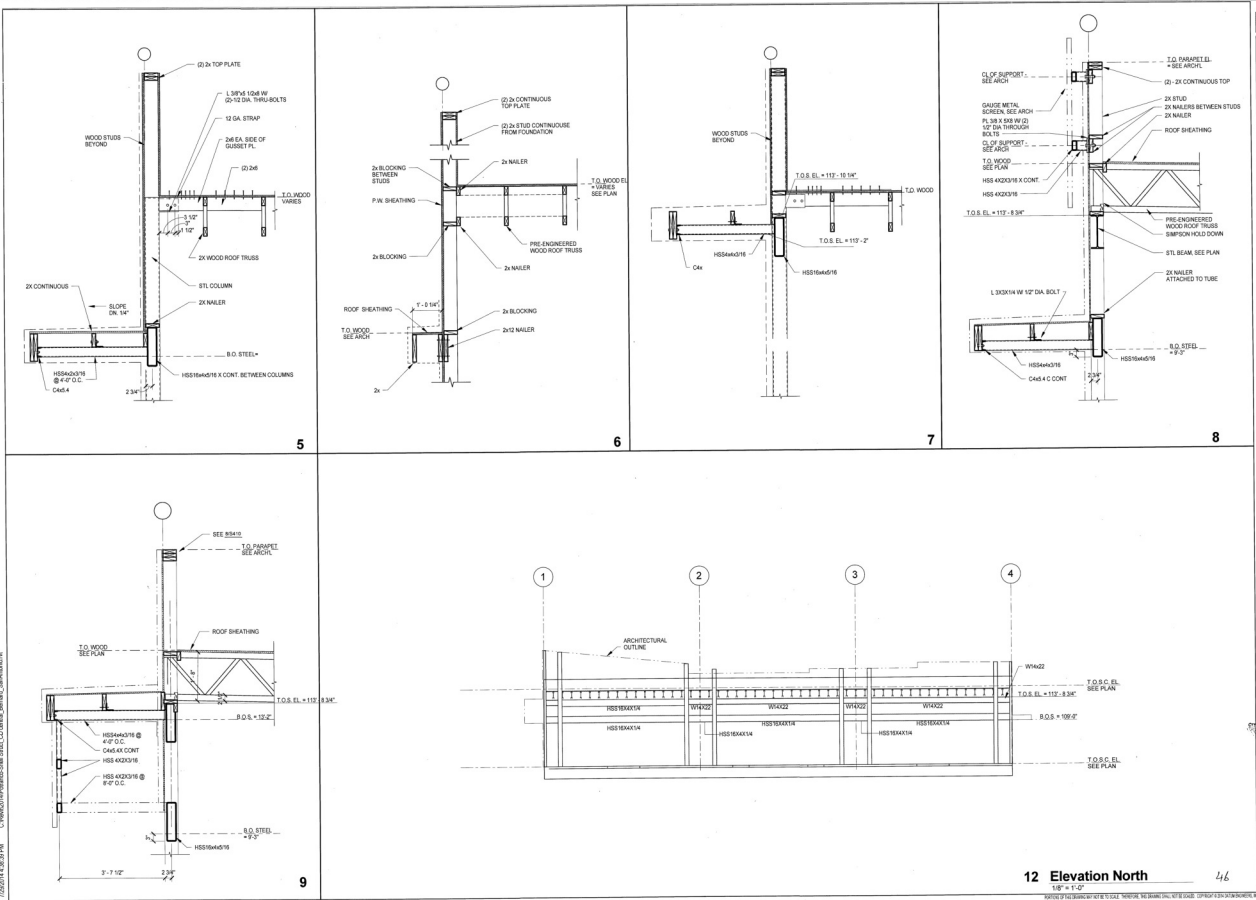
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DATE: 1/25/2014
Issue Date

PROJECT NO: 14-022
DRAWN BY: BR
REVIEW BY: LR
PROJECT ARCHITECT:
GABRIEL DURAND HOLLIS, FAIA

S410



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